

DRAWING INDEX		ISSUE LOG
PERMIT		
S001	DRAWING INDEX AND ABBREVIATIONS	X
S002	GENERAL STRUCTURAL NOTES	X
S003	GENERAL STRUCTURAL NOTES CONT. & SPECIAL INSPECTION NOTES	X
S004	SPECIAL INSPECTION NOTES CONT.	X
S005	SPECIAL INSPECTION NOTES CONT.	X
S401	DUMPSTER ENCLOSURE PLANS AND DETAILS	X
ISSUE LOG KEY:		
' X '	ISSUED AS PART OF A SET	
' - '	NOT A PART OF ISSUED SET	
' * '	FOR INFORMATION ONLY	
DATE		01/07/2026

## ABBREVIATIONS

A.B.	ANCHOR BOLT	FLR.	FLOOR	P/C	PRECAST
ACI	AMERICAN CONCRETE INSTITUTE	FT.	FOOT	PCF	POUNDS PER CUBIC FOOT
ADD'L.	ADDITIONAL	FTG.	FOOTING	PL	PLATE
AESS	ARCHITECTURAL EXPOSED STRUCTURAL STEEL	GA.	GAUGE	P.P.	PARTIAL PENETRATION
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION INCORPORATED	GALV.	GALVANIZED	PSI	POUNDS PER SQUARE INCH
ALT.	ALTERNATE	HORIZ.	HORIZONTAL	P/T	POST-TENSIONED
ALUM.	ALUMINUM	HSS	HOLLOW STRUCTURAL SECTION	PVC	POLYVINYL CHLORIDE
APA	AMERICAN PLYWOOD ASSOCIATION	IBC	INTERNATIONAL BUILDING CODE	R. RAD.	RADIUS
ARCH.	ARCHITECT	ICBO	INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS	RCSC	RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS	ICC	INTERNATIONAL CODE COUNCIL	REF.	REFERENCE
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	I.D.	INSIDE DIAMETER	RET.	RETURN
AWS	AMERICAN WELDING SOCIETY	IN.	INCH	REINF.	REINFORCING
BLDG.	BUILDING	INT.	INTERIOR	REQ'D.	REQUIRED
BOT.	BOTTOM	K	KIPS	REQ'MTS.	REQUIREMENTS
BRBF	BUCKLING RESTRAINED BRACED FRAME	KSF	KIPS PER SQUARE FOOT	SCHED.	SCHEDULE
C.G.	CENTER OF GRAVITY	KSI	KIPS PER SQUARE INCH	S.C.	SLIP CRITICAL
C.I.P.	CAST IN PLACE	LBS.	POUND	SIM.	SIMILAR
C.J.	CONTROL JOINT	L.L.	LIVE LOAD	SLRS	SEISMIC LOAD RESISTING SYSTEM
C.J.P.	COMPLETE JOINT PENETRATION	LLH	LONG LEG HORIZONTAL	S.O.G.	SLAB ON GRADE
CL	CENTERLINE	LLV	LONG LEG VERTICAL	SPEC.	SPECIFICATION
CLR.	CLEAR	LOC.	LOCATION	SQ.	SQUARE
CMU	CONCRETE MASONRY UNIT	LONG.	LONGITUDINAL	SS	STAINLESS STEEL
COL.	COLUMN	LVF	LOW VELOCITY FASTENER	SSMA	STEEL STUD MANUFACTURERS ASSOCIATION
CONC.	CONCRETE	MAX.	MAXIMUM	STD.	STANDARD
CONN.	CONNECTION	MBMA	METAL BUILDING MANUFACTURERS ASSOCIATION	STRUCT.	STRUCTURAL
CONST.	CONSTRUCTION	MECH.	MECHANICAL	SYM.	SYMMETRICAL
CONT.	CONTINUOUS	MFR.	MANUFACTURER	THRU.	THROUGH
$d_b$	BAR DIAMETER	MIN.	MINIMUM	T&G	TONGUE AND GROOVE
DBA	DEFORMED BAR ANCHOR	MISC.	MISCELLANEOUS	TJ	TRUSS JOIST
DET.	DETAIL	MPH	MILES PER HOUR	TRANS.	TRANSVERSE
DIA. Ø	DIAMETER	MT	MAGNETIC PARTICLE TESTING	TS	LIGHT GAUGE TUBE STEEL
DIAG.	DIAGONAL	(N)	NEW	TYP.	TYPICAL
D.L.	DEAD LOAD	N.I.C.	NOT IN CONTRACT	U.N.O.	UNLESS NOTED OTHERWISE
DWG.	DRAWING	NOM.	NOMINAL	UT	ULTRASONIC TESTING
ELEC.	ELECTRICAL	N.T.S.	NOT TO SCALE	VERT.	VERTICAL
EL.	ELEVATION	o.c.	ON CENTER	WF	WIDE FLANGE
EQ.	EQUAL	O.D.	OUTSIDE DIAMETER	w/o	WITHOUT
EXIST. (E)	EXISTING	OPP.	OPPOSITE	W.P.	WORK POINT
EXP.	EXPANSION	OWJ	OPEN WEB JOIST	WPS	WELDING PROCEDURE
EXT.	EXTERIOR	PAF	POWDER ACTUATED FASTENER	WWF	SPECIFICATION
FDN.	FOUNDATION	PART.	PARTITION		WELDED WIRE FABRIC
FIN.	FINISH				

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DRAWING INDEX  
AND  
ABBREVIATIONS

**S001**

## GENERAL STRUCTURAL NOTES

### GENERAL

STRUCTURAL DRAWINGS ARE A PART OF THE CONTRACT DOCUMENTS AND ARE COMPLEMENTARY TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING DRAWINGS, THE SPECIFICATIONS AND OTHER CONTRACT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE REQUIREMENTS FROM THE CONTRACT DOCUMENTS INTO THEIR SHOP DRAWINGS AND WORK. AS REQUIRED BY THE GENERAL CONDITIONS, THE CONTRACTOR SHALL PROMPTLY REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES, OR OMISSIONS IN THE CONTRACT DOCUMENTS DISCOVERED BY OR MADE KNOWN TO THE CONTRACTOR.

THE GENERAL STRUCTURAL NOTES SUPPLEMENT THE PROJECT SPECIFICATIONS. REFER TO THE PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS SHALL TAKE PRIORITY OVER THE GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK. WHERE CONFLICT EXISTS, THE MORE STRINGENT OR RESTRICTIVE REQUIREMENT SHALL GOVERN UNTIL CLARIFICATION IS REQUESTED.

#### CODE REQUIREMENTS:

CONFORM TO THE 2022 OREGON STRUCTURAL SPECIALTY CODE (OSSC), BASED ON THE 2021 INTERNATIONAL BUILDING CODE (IBC).

#### TEMPORARY CONDITIONS:

THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES UNTIL COMPLETION.

EXCAVATIONS SHALL NOT REDUCE THE VERTICAL OR LATERAL SUPPORT FOR ANY FOUNDATION OF THIS PROJECT OR ANY ADJACENT STRUCTURE WITHOUT FIRST UNDERPINNING OR PROTECTING THE FOUNDATION AGAINST DETERMINANT LATERAL AND/OR VERTICAL MOVEMENT. REF. SUBMITTALS SECTION FOR CONTRACTOR'S DELEGATED DESIGN RESPONSIBILITY WHERE SUCH SUPPORT IS REQUIRED.

#### EXISTING CONDITIONS:

ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.

#### ASSUMED FUTURE CONSTRUCTION:

VERTICAL: NONE  
HORIZONTAL: NONE

### DESIGN CRITERIA

DESIGN WAS BASED ON THE STRENGTH AND DEFLECTION CRITERIA OF THE OSSC. IN ADDITION TO THE DEAD LOADS, THE FOLLOWING LOADS AND ALLOWABLES WERE USED FOR DESIGN, WITH LIVE LOADS (L.L.) REDUCED PER OSSC:

GEOTECHNICAL CRITERIA		
DESIGN BASED ON REPORT BY:	CODE MINIMUM	
ALLOWABLE SOIL PRESSURE:		
ON NATIVE GRADE	1,500 PSF	
SHORT TERM LOADING	1/3 INCREASE	
WIND CRITERIA		
RISK CATEGORY	II	
BASIC WIND SPEED	VULT = 97 MPH (3-SECOND GUST)	VASD = 75 MPH (3-SECOND GUST)
EXPOSURE CATEGORY	B	
GUST / INTERNAL PRESSURE	GCpi = +/- 0.18	
EXTERIOR COMPONENT AND CLADDING DESIGN PRESSURES	Zone 4 +16/-16 PSF Zone 5 +16/-16.8 PSF	
SEISMIC CRITERIA		
RISK CATEGORY	II	
SEISMIC DESIGN CATEGORY	D	
SITE CLASS	C	
SEISMIC IMPORTANCE FACTOR	IE = 1.0	
MAPPED SPECTRAL ACCELERATION PARAMETERS	SS = 0.95	S1 = 0.35
DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS	SDS = 0.77	SD1 = 0.52
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE PER ASCE 7-22, SECTION 12.8	
	X DIRECTION (EAST / WEST)	Y DIRECTION (NORTH / SOUTH)
SEISMIC FORCE RESISTING SYSTEM (SFRS) REF. ASCE 7-16, TABLE 12.2-1	G1: STEEL SPECIAL CANTILEVER COLUMN SYSTEMS	G1: STEEL SPECIAL CANTILEVER COLUMN SYSTEMS
RESPONSE MODIFICATION FACTOR	R = 2.5	R = 2.5
SEISMIC RESPONSE COEFFICIENT	Cs = 0.293	Cs = 0.293
DESIGN BASE SHEAR	1 KIPS	1 KIPS
REDUNDANCY FACTOR	rho = 1.3	rho = 1.3
DESIGN INELASTIC STORY DRIFT	Delta = 2.00"	Delta = 2.00"

### STRUCTURAL OBSERVATIONS

THE STRUCTURAL ENGINEER OF RECORD (SEOR) WILL PERFORM STRUCTURAL OBSERVATIONS BASED ON THE REQUIREMENTS OF THE OSSC AT THE STAGES OF CONSTRUCTION LISTED BELOW. CONTRACTOR SHALL PROVIDE SUFFICIENT ADVANCED NOTICE AND ACCESS FOR THE SEOR TO PERFORM THESE OBSERVATIONS.

ITEM	COMMENTS
PRIOR TO FIRST CONCRETE POUR	AFTER REBAR PLACEMENT
DURING INITIAL STEEL ERECTION	
AS REQUIRED TO ADDRESS STRUCTURAL ISSUES	

A FIELD REPORT WILL BE SUBMITTED TO THE BUILDING DEPARTMENT FOLLOWING EACH SITE VISIT.

STRUCTURAL OBSERVATION IS FOR THE GENERAL CONFORMANCE OF THE STRUCTURAL DRAWINGS AND DOES NOT ALLEVIATE ANY SPECIAL INSPECTION REQUIREMENTS.

### SPECIAL INSPECTIONS AND TESTING

SPECIAL INSPECTION WILL BE PROVIDED BY THE OWNER BASED ON THE REQUIREMENTS OF THE OSSC AS SUMMARIZED IN THE SPECIAL INSPECTION AND TESTING PROGRAM. CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SPECIAL INSPECTOR TO PERFORM THESE INSPECTIONS.

### SUBMITTALS

SUBMIT SHOP DRAWINGS AND OTHER SUBMITTALS TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION AND CONSTRUCTION OF STRUCTURAL ITEMS. IF THE SUBMITTALS DIFFER FROM OR ADD TO THE STRUCTURAL CONTRACT DOCUMENTS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF OREGON. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ARE SUBJECT TO REVIEW AND ACCEPTANCE BY THE SEOR.

FIELD ENGINEERED DETAILS DEVELOPED BY THE CONTRACTOR THAT DIFFER FROM OR ADD TO THE STRUCTURAL DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OREGON AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO CONSTRUCTION.

THE USE OF REPRODUCTIONS OR PHOTOCOPIES OF THE CONTRACT DRAWINGS SHALL NOT BE PERMITTED. WHEN CAD OR REVIT FILES ARE PROVIDED TO THE CONTRACTOR OR SUBCONTRACTORS, IT IS THE RESPONSIBILITY OF THE CONTRACTOR/SUBCONTRACTOR TO REMOVE ALL INFORMATION NOT DIRECTLY RELEVANT TO THE SCOPE OF THE SUBMITTAL AS WELL AS ALL REFERENCES TO OUTSIDE SOURCE FILES.

DELEGATED DESIGN SUBMITTALS SHALL INCLUDE DESIGN DRAWINGS AND CALCULATIONS FOR ITEMS THAT ARE DESIGNED BY OTHERS. DELEGATED DESIGN SUBMITTALS SHALL BEAR THE SEAL AND SIGNATURE OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OREGON ON EVERY DRAWING SHEET AND ON THE CALCULATION COVER SHEET, AND SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION. CALCULATIONS AND DETAILS SHALL BE INCLUDED FOR ALL CONNECTIONS TO THE STRUCTURE, CONSIDERING LOCALIZED EFFECTS ON STRUCTURAL ELEMENTS. DESIGN SHALL BE BASED ON THE REQUIREMENTS OF THE OSSC AND AS NOTED UNDER "DESIGN CRITERIA".

SUBMITTALS AND DELEGATED DESIGN SUBMITTALS SHALL INCLUDE THE FOLLOWING:

ITEM	SUBMITTAL	DELEGATED DESIGN SUBMITTAL	COMMENTS
CONCRETE MIX DESIGNS	X		REF. TABLE NOTE 3 AND 4
CONCRETE REINFORCEMENT	X		REF. TABLE NOTE 4
CONCRETE ANCHORAGES	X		REF. TABLE NOTE 4
EMBEDDED STEEL ITEMS	X		REF. TABLE NOTE 4
STRUCTURAL STEEL	X		REF. TABLE NOTE 4
STEEL WELDING PROCEDURES	X		

#### TABLE NOTES:

- CONTRACTOR SHALL ENGAGE A PROFESSIONAL ENGINEER TO PREPARE AN ASSESSMENT OF ANY EXCAVATIONS THAT MAY REDUCE THE VERTICAL OR LATERAL SUPPORT OF AN EXISTING FOUNDATION AS REQUIRED BY OSSC SECTION 1803.5.7. THE ASSESSMENT SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND SHALL INCLUDE DETAILS AND SEQUENCING FOR CONSTRUCTION OF ANY UNDERPINNING OR BRACING THAT IS REQUIRED.

### CONCRETE MIX DESIGNS

CONCRETE WORK SHALL CONFORM TO CHAPTER 19 OF THE OSSC. CONCRETE STRENGTHS SHALL BE VERIFIED BY STANDARD CYLINDER TESTS PER ASTM C39. CONCRETE MIX TO BE DESIGNED AND PROPORTIONED BY THE CONTRACTOR IN ACCORDANCE WITH ACI 318-19 CHAPTER 26, ACI 301-16 SECTION 4 AND THE FOLLOWING INFORMATION:

MIX TYPE	USE	f <sub>c</sub> (PSI)	TEST AGE (DAYS)	MAX. W/C/M RATIO	MAX. AGG. SIZE	EXPOSURE CLASS
E	SPREAD FOOTINGS AND EXPOSED SLAB ON GRADE	4,000	28	N/A	1"	F0 S0 W0 C0

#### TABLE NOTES:

- REF. ACI 318-19 TABLE 19.3.2.1 FOR ADDITIONAL MIX REQUIREMENTS SPECIFIC TO EXPOSURE CLASS.
- ALL CONCRETE MIXES TO BE NORMAL WEIGHT CONCRETE, U.N.O.
- EXPOSURE CATEGORY "F" APPLIES TO LEVEL OF FREEZE THAW EXPOSURE.
- EXPOSURE CATEGORY "S" APPLIES TO REQUIRED LEVEL OF SULFATE EXPOSURE.
- EXPOSURE CATEGORY "W" APPLIES TO REQUIRED LEVEL OF PERMEABILITY.
- EXPOSURE CATEGORY "C" APPLIES TO CORROSIVE LOCATIONS - INCLUDING SURROUNDING ENVIRONMENT (SUCH AS MARINE ENVIRONMENT) AND CORROSIVE SOILS.
- ESTABLISH WATER-CEMENTITIOUS MATERIAL RATIO PER ACI 301-16 SECTION 4.

PORTLAND CEMENT CONTENT MAY BE REPLACED WITH FLY ASH CONFORMING TO ASTM C618 (INCLUDING TABLE 2A) TYPE F OR TYPE C, SLAG CEMENT CONFORMING TO ASTM C989, AND SILICA FUME CONFORMING TO ASTM C1240 PROVIDED THAT THE MIX STRENGTH IS SUBSTANTIATED BY TEST DATA.

FOR MIX DESIGNS WITH f<sub>c</sub> = 5,000 PSI OR LESS, SLAG CEMENT MAY BE SUBSTITUTED FOR FLY ASH AT A 1:1 RATIO WITHOUT TEST DATA. WHEN SLAG CEMENT IS SUBSTITUTED IN HIGHER STRENGTH MIXES OR AT A DIFFERENT RATIO, THE MIX STRENGTH MUST BE SUBSTANTIATED BY TEST DATA.

ALL CONCRETE SUBJECT TO EXPOSURE CLASSES F1, F2 OR F3 SHALL BE AIR ENTRAINED. AIR-ENTRAINING AGENTS SHALL CONFORM TO ASTM C260. THE AMOUNT OF ENTRAINED AIR SHALL BE ACCORDING TO ACI 318-19 TABLE 19.3.3.1 AS INDICATED BELOW WITH A FIELD TOLERANCE OF +/- 1.5 PERCENT BY VOLUME. THE AMOUNT OF ENTRAINED AIR SHALL BE MEASURED IN THE FIELD AT THE DISCHARGE FROM THE TRUCK.

CONCRETE MIX AIR CONTENT		
MAX. AGGREGATE SIZE	CONCRETE SUBJECT TO FREEZE/THAW (EXPOSURE CLASS F1)	CONCRETE SUBJECT TO CONT. MOISTURE AND/OR DEICING CHEMICALS (EXPOSURE CLASS F2 AND F3)
1"	4.5%	6.0%

A WATER-REDUCING ADMIXTURE CONFORMING TO ASTM C494 USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS SHALL BE INCORPORATED IN CONCRETE MIX DESIGNS. A HIGH-RANGE WATER-REDUCING (HRWR) ADMIXTURE CONFORMING TO ASTM C494 TYPE F OR G MAY BE USED IN CONCRETE MIXES PROVIDING THAT THE SLUMP DOES NOT EXCEED 10".

THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS AND TEST DATA COMPLIANT WITH ACI 301-16 AND ACI 318-19 A MINIMUM OF TWO WEEKS PRIOR TO PLACING CONCRETE. NO WATER MAY BE ADDED TO CONCRETE IN THE FIELD UNLESS SPECIFICALLY APPROVED IN WRITING BY THE CONCRETE SUPPLIER AND SEOR IN CONJUNCTION WITH THE CONCRETE MIX DESIGN.

### CONCRETE REINFORCING STEEL

CONCRETE REINFORCEMENT SHALL BE AS LISTED BELOW. ASTM A615 REINFORCEMENT MAY BE SUBSTITUTED FOR ASTM A706 REINFORCEMENT PROVIDED THAT THE ACTUAL YIELD STRENGTH BASED ON MILL TESTS DOES NOT EXCEED f<sub>y</sub> BY MORE THAN 18,000 PSI AND THE RATIO OF ACTUAL TENSILE STRENGTH TO ACTUAL YIELD STRENGTH IS NOT LESS THAN 1.25 AND THE ELONGATION REQUIREMENTS OF ASTM A706 ARE MET PER ACI 318-19 SECTION 20.2.2.5. MILL TESTS CERTIFICATIONS FOR SUBSTITUTED BARS SHALL BE SUBMITTED TO THE SPECIAL INSPECTOR AND SEOR PRIOR TO PLACEMENT. ASTM A706 REINFORCEMENT MAY BE SUBSTITUTED FOR ASTM A615 REINFORCEMENT.

REINFORCING LOCATION	MATERIAL GRADE
ALL OTHER USES U.N.O.	ASTM A615 GRADE 60

ALL REINFORCING STEEL SHALL BE SECURELY TIED IN PLACE WITH #16 ANNEALED IRON WIRE. BARS IN BEAMS AND SLABS SHALL BE SUPPORTED ON WELL-CURED CONCRETE BLOCKS OR APPROVED METAL OR PLASTIC CHAIRS, AS SPECIFIED BY THE CRSI MANUAL OF STANDARD PRACTICE, MSP-1. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH ACI MNL-66 "ACI DETAILING MANUAL". SHOP DRAWINGS SHALL INCLUDE ELEVATIONS OF ALL BEAMS, WALLS AND COLUMNS SHOWING BAR LOCATIONS.

REINFORCING BARS SHALL NOT BE BENT OR STRAIGHTENED IN THE FIELD WITHOUT APPROVAL OF THE SEOR. PREHEATING METHODS SHALL BE SUBMITTED TO THE SEOR FOR APPROVAL PRIOR TO BENDING OF BARS #6 OR LARGER.

LAP ALL REINFORCING BARS PER THE TYPICAL LAP SPLICE LENGTH SCHEDULES, EXCEPT AS NOTED ON DRAWINGS. USE LAP LENGTH FOR SMALLER BAR WHEN SPLICING DIFFERENT BAR SIZES. BARS SPLICED WITH NONCONTACT SPLICES SHALL BE SPACED NO FARTHER THAN 1/5TH THE LAP LENGTH OR 6 INCHES. MECHANICAL SPLICES NOTED ON THE PLANS SHALL BE DAYTON SUPERIOR BAR-LOCK OR TAPER-LOCK COUPLERS (UES ER-319) OR APPROVED EQUAL WITH A CURRENT EVALUATION REPORT. FOR CONCRETE STRENGTHS BETWEEN TABULATED VALUES, USE LAP SPLICE LENGTHS FOR NEAREST LOWER CONCRETE STRENGTH INDICATED.

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## GENERAL STRUCTURAL NOTES CONT.

### CONCRETE REINFORCING STEEL

TYP. FOUNDATION LAP SPLICE LENGTH SCHEDULE (IN.) - 60 KSI												
BAR SIZE	BOTTOM BARS					TOP BARS						
	3,000 PSI	4,000 PSI	5,000 PSI	6,000 PSI	7,000 PSI	8,000 PSI	3,000 PSI	4,000 PSI	5,000 PSI	6,000 PSI	7,000 PSI	$\geq 8,000$ PSI
#4	22	20	18	16	14	14	28	26	22	20	20	18
#5	28	24	22	20	18	18	36	32	28	26	24	22
#6	34	28	26	24	22	22	42	38	34	30	28	28
#7	48	42	38	34	32	30	62	54	48	44	42	38
#8	54	48	42	38	36	34	70	62	54	50	46	44

#### TABLE NOTES:

1. SPLICE LENGTHS APPLY TO ASTM A615 OR ASTM A706 GRADE 60 DEFORMED REINFORCING BARS ONLY.
2. SPLICE LENGTHS ARE BASED ON THE CLEAR COVER AND MINIMUM BAR CLEAR SPACING INDICATED BELOW.
3. SLAB, FOUNDATION AND MAT TOP BARS ARE BARS CAST ABOVE MORE THAN 12" OF FRESH CONCRETE. ALL OTHER SLAB BARS MAY BE CONSIDERED BOTTOM BARS.

REINFORCING STEEL SHALL HAVE PROTECTION AND SPACING AS FOLLOWS:

CONCRETE COVER		
USE	CLEAR COVER	MIN. CLEAR SPACING
CONCRETE EXPOSED TO EARTH OR WEATHER	1-1/2" (#5 AND SMALLER) 2" (#6 AND LARGER)	2db OR 1"
CONCRETE CAST AGAINST AND EXPOSED TO EARTH	3"	3db OR 1"

### STRUCTURAL STEEL

STRUCTURAL STEEL SHALL BE OF THE MATERIAL AND TYPE LISTED BELOW, U.N.O.:

STRUCTURAL STEEL	
SHAPE	MATERIAL GRADE
PLATES, U.N.O.	ASTM A572, GRADE 50
HOLLOW STRUCTURAL SECTIONS (RECTANGULAR)	ASTM A1085, GRADE A (FY=50KSI)

DESIGN, DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE AISC 360, "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" WITH "COMMENTARY" AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", WITH THE FOLLOWING CLARIFICATIONS AND ADDITIONS:

1. CLARIFY SECTIONS 7.5.1 AND 7.5.3 AS FOLLOWS:  
EMBEDMENT LOCATION DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR INFORMATION ONLY. THE SEOR IS NOT RESPONSIBLE FOR THE APPROVAL OF EMBEDMENT LOCATION DRAWINGS.
2. ADD THE FOLLOWING PARAGRAPH TO SECTION 7.10.3:  
"THE ERECTOR SHALL HAVE THE SOLE RESPONSIBILITY FOR DETERMINING THE MEANS AND METHODS USED TO PROPERLY AND ADEQUATELY BRACE THE FRAMING DURING ERECTION."

BOLTS SHALL CONFORM TO THE ASTM AND RCSC SPECIFICATIONS FOR JOINTS USING HIGH STRENGTH BOLTS. BOLTS SHALL BE ASTM F3125 GRADE A325 AND GRADE A490 WHERE NOTED, AND SNUG-TIGHT UNLESS NOTED OTHERWISE.

WELDING SHALL CONFORM TO THE AWS CODES FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS D1.1 AND APPROVED BY THE STRUCTURAL ENGINEER. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER-METAL MANUFACTURER.

WELDS SHALL BE MADE USING E70XX ELECTRODES AND SHALL BE 3/16" MINIMUM, UNLESS OTHERWISE NOTED. WELDING SHALL BE BY AWS CERTIFIED WELDERS.

PROVIDE WEEP HOLES AT EXTERIOR CLOSED SECTIONS WHERE MOISTURE MAY ACCUMULATE. LOCATE WEEP HOLES AT BOTTOM OF HORIZONTAL MEMBERS AT MIDSPAN UNLESS OTHER NOTED. LOCATE WEEP HOLES AT BOTTOM OF VERTICAL MEMBERS EXCEPT AT ROOF ASSEMBLIES. ALL WEEP HOLES TO BE APPROVED PRIOR TO FABRICATION.

NON-SHRINK GROUT USED UNDER BEARING AND BASE PLATES SHALL BE ASTM C 1107. FACTORY-PACKAGED, NONMETALLIC AGGREGATE GROUT, NONCORROSIVE, NONSTAINING, MIXED WITH WATER TO CONSISTENCY SUITABLE FOR APPLICATION AND A 30-MINUTE WORKING TIME. GROUT STRENGTH SHALL BE 8,000 PSI MINIMUM AT 28 DAYS.

DISSIMILAR METALS SHALL BE SEPARATED AS REQUIRED TO PREVENT GALVANIC CORROSION BY COMPLETELY COVERING CONTACT AREAS WITH HESKINS 3453 CORROSION PROTECTION TAPE OR APPROVED EQUAL MATERIAL.

### GALVANIZING AND DUPLEX COATING

ALL STEEL EXPOSED TO WEATHER OR LOCATED OUTSIDE THE BUILDING ENVELOPE SHALL BE HOT-DIP GALVANIZED UNLESS NOTED OTHERWISE IN PROJECT SPECIFICATIONS OR DRAWINGS. WHERE THESE ELEMENTS ARE ALSO EXPOSED TO VIEW THEY SHALL ADDITIONALLY BE PAINTED OR POWDER COATED PER SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.

CONTRACTOR TO COMMUNICATE WITH GALVANIZER FOR THE PROJECT EARLY ON TO INFORM THE GALVANIZER THAT THE STEEL IS TO RECEIVE A DUPLEX COATING. HOT DIPPED GALVANIZED STEEL THAT IS TO BE PAINTED SHALL BE PREPARED PER ASTM D6386. HOT DIPPED GALVANIZED STEEL THAT IS TO BE POWDER COATED SHALL BE PREPARED PER ASTM D7803.

ALL GALVANIZED STEEL IS TO BE DETAILED TO BE SHOP WELDED AND FIELD BOLTED. WHERE FIELD WELDING IS REQUIRED DUE TO FIELD CONDITIONS, REPAIR DAMAGED GALVANIZED COATING WITH ZINC RICH PAINT PER ASTM A780 WITH EFFECTIVE THICKNESS EQUAL TO HOT-DIP GALVANIZED COATING.

### SPECIAL INSPECTIONS AND TESTING

#### STATEMENT OF SPECIAL INSPECTION NOTES:

1. SPECIAL INSPECTIONS SHALL CONFORM TO SECTION 1705 OF THE 2022 OSSC, CONTRACT DOCUMENTS AND APPROVED SUBMITTALS. REFER TO SPECIAL INSPECTION AND TESTING TABLES FOR PROJECT REQUIREMENTS.
2. SPECIAL INSPECTIONS AND ASSOCIATED TESTING SHALL BE PERFORMED BY AN APPROVED ACCREDITED INDEPENDENT AGENCY MEETING THE REQUIREMENTS OF ASTM E329 (MATERIALS). THE INSPECTION AND TESTING AGENCY SHALL FURNISH TO THE STRUCTURAL ENGINEER A COPY OF THEIR SCOPE OF ACCREDITATION. SPECIAL INSPECTORS SHALL BE APPROVED BY THE BUILDING OFFICIAL. WELDING INSPECTORS SHALL BE QUALIFIED PER SECTION 6.1.4.1(1) OF AWS D1.1.
3. THE SPECIAL INSPECTOR SHALL OBSERVE THE INDICATED WORK FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION AND NOTED IN THE INSPECTION REPORTS.
4. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS FOR EACH INSPECTION TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER, CONTRACTOR, AND OWNER. THE SPECIAL INSPECTION AGENCY SHALL SUBMIT A FINAL REPORT STATING THAT THE WORK REQUIRING SPECIAL INSPECTION WAS INSPECTED AND IS IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THAT ALL DISCREPANCIES NOTED IN THE INSPECTION REPORTS HAVE BEEN CORRECTED.
5. QUALITY ASSURANCE (QA) IS REQUIRED FOR STRUCTURAL STEEL ITEMS PER AISC 360 AND 341 UNLESS SPECIFICALLY NOTED OTHERWISE. QUALITY CONTROL (QC) TO BE PROVIDED BY THE FABRICATOR, ERECTOR OR OTHER RESPONSIBLE CONTRACTOR AS APPLICABLE. CONTRACTOR AND SPECIAL INSPECTOR TO DOCUMENT QUALITY CONTROL AS REQUIRED IN AISC 360 SECTION N3 AND AISC 341 SECTION J2.
6. **INSPECTION TYPES:**
  - CONTINUOUS : THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
  - PERIODIC : THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.
  - OBSERVE : OBSERVE THESE FUNCTIONS ON A RANDOM, DAILY BASIS. OPERATIONS NEED NOT BE DELAYED PENDING OBSERVATIONS.
  - PERFORM : INSPECTIONS SHALL BE PERFORMED PRIOR TO THE FINAL ACCEPTANCE OF THE ITEM.
7. PERFORM INSPECTION PRIOR TO FINAL ACCEPTANCE OF THE ITEM FOR TEN WELDS TO BE MADE BY A GIVEN WELDER, WITH THE WELDER DEMONSTRATING UNDERSTANDING OF REQUIREMENTS AND POSSESSION OF SKILLS AND TOOLS TO VERIFY THESE ITEMS. THE PERFORM DESIGNATION OF THIS TASK SHALL BE REDUCED TO OBSERVE, AND THE WELDER SHALL PERFORM THIS TASK. SHOULD THE INSPECTOR DETERMINE THAT THE WELDER HAS DISCONTINUED PERFORMANCE OF THIS TASK, THE TASK SHALL BE RETURNED TO PERFORM UNTIL SUCH TIME AS THE INSPECTOR HAS RE-ESTABLISHED ADEQUATE ASSURANCE THAT THE WELDER WILL PERFORM THE INSPECTION TASKS LISTED.
8. SPECIAL INSPECTION OF MECHANICAL POST INSTALLED ANCHORS SHALL BE IN STRICT CONFORMANCE WITH THE ICC REPORT AND MANUFACTURER'S INSTALLATION REQUIREMENTS. ANCHOR INSTALLERS SHALL BE QUALIFIED AS REQUIRED BY JURISDICTION REQUIREMENTS.
  - INSPECTION REPORTS SHALL IDENTIFY NAMES OF INSTALLERS.
  - SPECIAL INSPECTOR SHALL PROVIDE DOCUMENTATION AT THE END OF ANCHOR INSTALLATIONS STATING THAT THE ANCHORS WERE INSPECTED PER APPROVED ANCHOR EVALUATION REPORT.
9. **TESTING ABBREVIATIONS:**
  - NDT - NON-DESTRUCTIVE TESTING
  - C.J.P. - COMPLETE JOINT PENETRATION
  - MT - MAGNETIC PARTICLE TESTING
  - RBS - REDUCED BEAM SECTION
10. DOCUMENT (D): INDICATES CONTRACTOR AND SPECIAL INSPECTOR TO PROVIDE DOCUMENTATION IN ACCORDANCE WITH AISC 341.

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**GENERAL STRUCTURAL NOTES CONT. & SPECIAL INSPECTIONS**

**S003**

## SPECIAL INSPECTIONS CONT.

GENERAL - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
FABRICATORS	1705.11 1704.2.5				SPECIAL INSPECTION IS REQUIRED FOR STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES FABRICATED ON THE PREMISES OF A FABRICATOR'S SHOP, SPECIAL INSPECTIONS SHALL BE PERFORMED DURING FABRICATION. PERFORMING SPECIAL INSPECTIONS IS NOT REQUIRED, WHERE FABRICATOR HAS BEEN APPROVED AS AN APPROVED FABRICATOR, PER SECTION 1704.2.5.1.
DEFERRED SUBMITTALS				X	SPECIAL INSPECTION REQUIREMENTS FOR DEFERRED SUBMITTAL ITEMS, INCLUDING REQUIREMENTS FOR DESIGNATED SEISMIC SYSTEMS IN ACCORDANCE WITH OSSC SECTION 1705.13.4 IF APPLICABLE, TO BE SPECIFIED BY THE SYSTEM ENGINEER AND INCLUDED WITH DEFERRED SUBMITAL DOCUMENTS.
SUBMITTALS TO THE BUILDING OFFICIAL	1704.5			X	CERTIFICATES OF COMPLIANCE, REPORTS OF PRE-CONSTRUCTION TESTS, OR REPORTS OF MATERIAL PROPERTIES SHALL BE SUBMITTED TO THE BUILDING OFFICIAL.
POST INSTALLED MECHANICAL ANCHORS AND ADHESIVE ANCHORS (EXCLUDING CONDITIONS NOTED ABOVE) IN HARDENED CONCRETE				X	

SOILS/GEOTECHNICAL - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARDS REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY				X	
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL				X	
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS				X	
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY				X	

SOILS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARDS REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY				X	
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL				X	
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS				X	
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY				X	

CONCRETE - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)		REMARKS
			CONTINUOUS	PERIODIC	
GENERAL	1705.3 1901.6	ACI 318: 26.13			SPECIAL INSPECTIONS OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1705.3 OF THE IBC AND SECTION 26.13 OF ACI 318.
REINFORCING STEEL PLACEMENT	1901.5	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3, 26.13.3.3		X	REINFORCING TO COMPLY WITH ALL CODE PROTECTION, SPACING AND TOLERANCE LIMITS.
INSPECT ANCHORS/BOLTS CAST IN CONCRETE	-	ACI 318: 26.13.3.3(g)	X	X	ALL CAST-IN-PLACE ANCHORS/BOLTS SHALL BE VISUALLY INSPECTED. REFERENCE STEEL INSPECTIONS FOR ADDITIONAL INSTALLATION, MATERIAL AND WELDING INSPECTIONS OF STEEL ITEMS EMBEDDED IN CONCRETE (HEADED STUDS, DBA's, ETC.)
VERIFYING USE OF REQUIRED MIX DESIGN(S)	1904.1 1904.2 1908	ACI 318: CH. 19, 26.4.3, 26.4.4		X	
CONCRETE SPECIMENS FOR TESTING		ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	X		PRIOR TO CONCRETE PLACEMENT, FABRICATE CONCRETE SPECIMENS FOR TESTING. SEE THE CONCRETE TESTING TABLE FOR ADDITIONAL INFORMATION.
CONCRETE PLACEMENT, NON-SHRINK GROUT	1908	ACI 318: 26.5, 26.13.3.2(a)	X		
CONCRETE CURING	1908.1	ACI 318: 26.5.3 - 26.5.5, 26.13.3		X	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURES AND TECHNIQUES
VERIFICATION OF FORMWORK		ACI 318: 26.11.1.2(b), 26.13.3		X	SPECIAL INSPECTIONS APPLY TO SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED
EMBEDDED ITEMS IN CONCRETE				X	ALL NON-STRUCTURAL EMBEDDED ITEMS, SUCH AS CONDUITS, PIPES AND SLEEVES, SHALL BE REVIEWED FOR CONFORMANCE WITH STRUCTURAL DOCUMENTS FOR SIZE, SPACING, LOCATION, EDGE DISTANCE AND TRIM REINFORCING.

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**SPECIAL INSPECTIONS CONT.**

**S004**

## SPECIAL INSPECTIONS CONT.

STEEL - SPECIAL INSPECTIONS					
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	INSPECTION (NOTES 5 AND 6)		REMARKS
			CONTINUOUS/ PERFORM	PERIODIC/ OBSERVE	
STEEL FABRICATION					
FABRICATION OF STRUCTURAL ELEMENTS	1704.2.5.1	AISC 360		X	REFER TO INSPECTION OF FABRICATOR REQUIREMENTS
MATERIAL VERIFICATION OF STRUCTURAL STEEL COMPONENTS	1705.2.1 TABLE 1705.2-3	ASTM A6 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS AISC 360 A3.1 AISC 360 N2.1 AISC 360 N3.2		X	CERTIFIED MILL TEST REPORTS IDENTIFICATION MARKINGS TO CONFORM TO AISC 360 AND ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS
MATERIAL VERIFICATION OF ANCHOR BOLTS AND THREADED RODS		AISC 360 A3.4 AISC 360 N3.2 ASTM STANDARDS SPECIFIED IN CONSTRUCTION DOCUMENTS		X	MANUFACTURER'S CERTIFIED TEST REPORTS
MATERIAL VERIFICATION OF WELD FILLER METALS	1705.2.1.1 TABLE 1705.2-5	AISC 360 A3.5 AISC 360 N3.2 APPLICABLE AWS A5 DOCUMENTS		X	MANUFACTURER'S CERTIFICATE OF COMPLIANCE IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS
STRUCTURAL STEEL WELDING					
VERIFYING USE OF PROPER WPS'S	1705.2.1 AWS D1.1	AISC 360 N3.2			RETAIN A RECORD OF WELDING PROCEDURE SPECIFICATIONS
VERIFYING WELDER QUALIFICATIONS		AWS D1.1		X	RETAIN A RECORD OF QUALIFICATION CARDS
COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS			X		
MULTIPASS FILLET WELDS			X		
SINGLE PASS FILLET WELDS GREATER THAN 5/16"			X		ALL WELDS VISUALLY INSPECTED PER AWS D1.16.9
PLUG AND SLOT WELDS			X		
SINGLE PASS FILLET WELDS LESS THAN OR EQUAL TO 5/16"			X		
SNUG-TIGHT BOLT INSTALLATION	1705.2.1 TABLE 1705.2 AISC 360 M2.5 AISC 360 N5.6	RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS SECTION 9 AISC 360 SECTION M2.5		X	ALL CONNECTIONS VISUALLY INSPECTED AND VERIFIED SNUG
INSPECTION TASKS PRIOR TO BOLTING					
MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS			X		
FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS			X		
CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH, IF THREADS ARE TO BE EXCLUDED FROM THE SHEAR PLANE)			X		
CORRECT BOLTING PROCEDURE SELECTED FOR JOINT DETAIL			X		
CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS			X		
PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED			X		
PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS			X		
INSPECTION TASKS DURING BOLTING					
FASTENER ASSEMBLIES PLACED IN ALL HOLES AND WASHERS AND NUTS ARE POSITIONED AS REQUIRED			X		
JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION			X		
FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING			X		
FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES			X		
INSPECTION TASKS AFTER BOLTING					
DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	1705.2.1.2 TABLE 1705.2	AISC 360 TABLE N5.6-3	X		

CONCRETE - TESTING				
SYSTEM OR MATERIAL	OSSC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY (NOTE 6)	REMARKS
CONCRETE STRENGTH	1705.3 C172	ASTM C39 ASTM C 31		
CONCRETE SLUMP		ASTM C143		
CONCRETE AIR CONTENT		ASTM C231		
CONCRETE TEMPERATURE		ASTM C1064		
			EACH 150 CY NOR LESS THAN EACH 5000 SF OF SLAB OR WALL PLACED EACH SHIFT	FABRICATE SPECIMENS AT TIME FRESH CONCRETE IS PLACED

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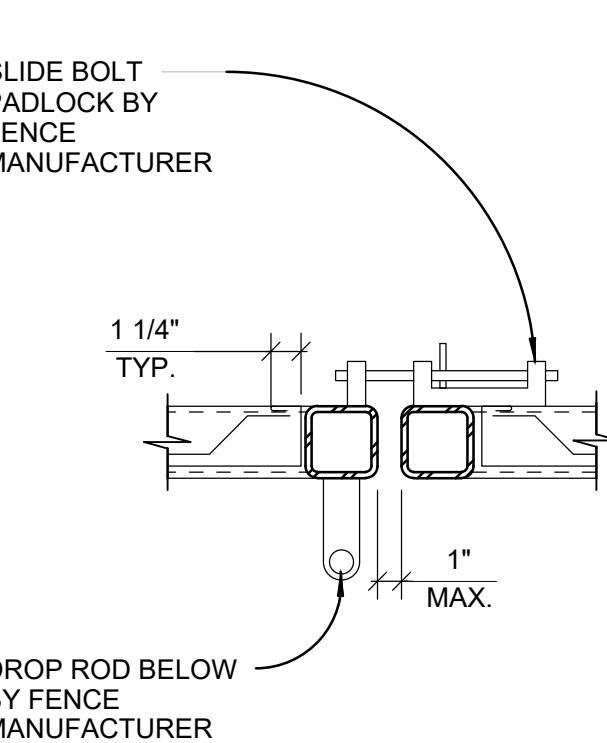
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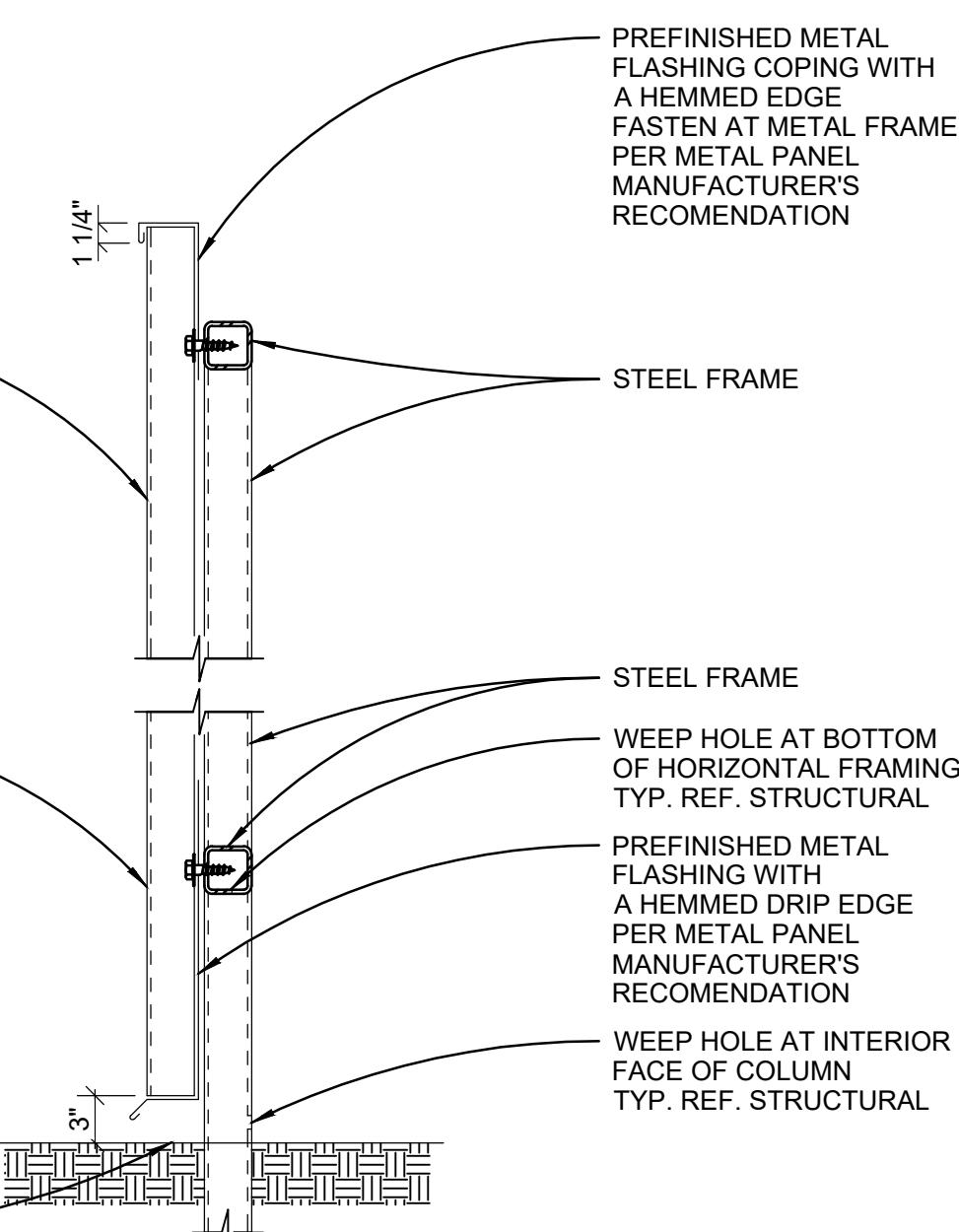
SPECIAL  
INSPECTIONS  
CONT.

**S005**

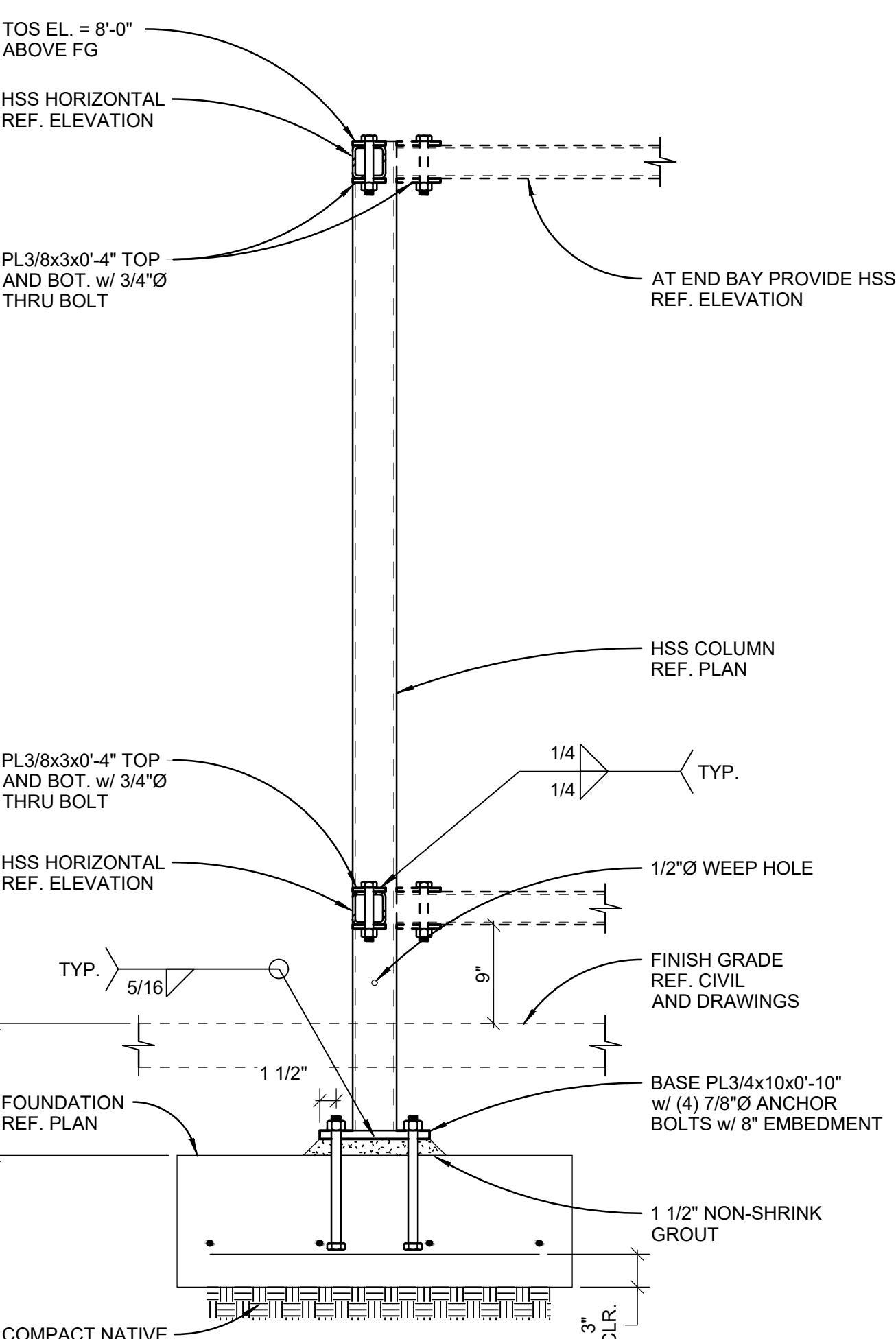


PLAN VIEW

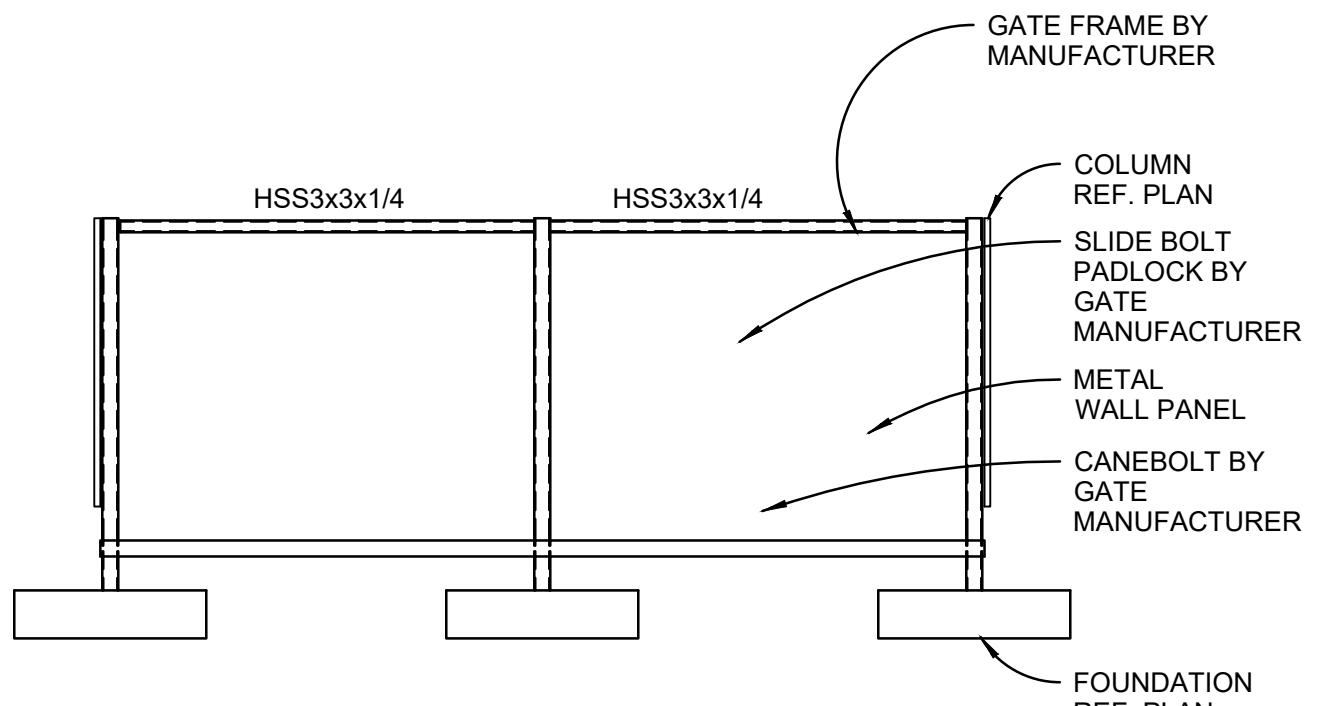
7 GATE DETAIL  
1 1/2" = 1'-0"



**NOTES:**  
METAL PANEL MANUFACTURER TO PROVIDE COMPLETE SCREENING SYSTEM WITH THE FOLLOWING:  
1. VERTICALLY ORIENTED METAL PANELS, PAINTED ON BOTH SIDES.  
2. EXPOSED FASTENERS. CUT AND GRIND DOWN ALL EXPOSED FASTENER ENDS.  
3. CLOSURE PLATE OR EDGING AT ALL CORNERS AND ENDS.  
4. ALL EXPOSED METAL TO BE HPC-3 OR PREFINISHED EQUIVALENT.

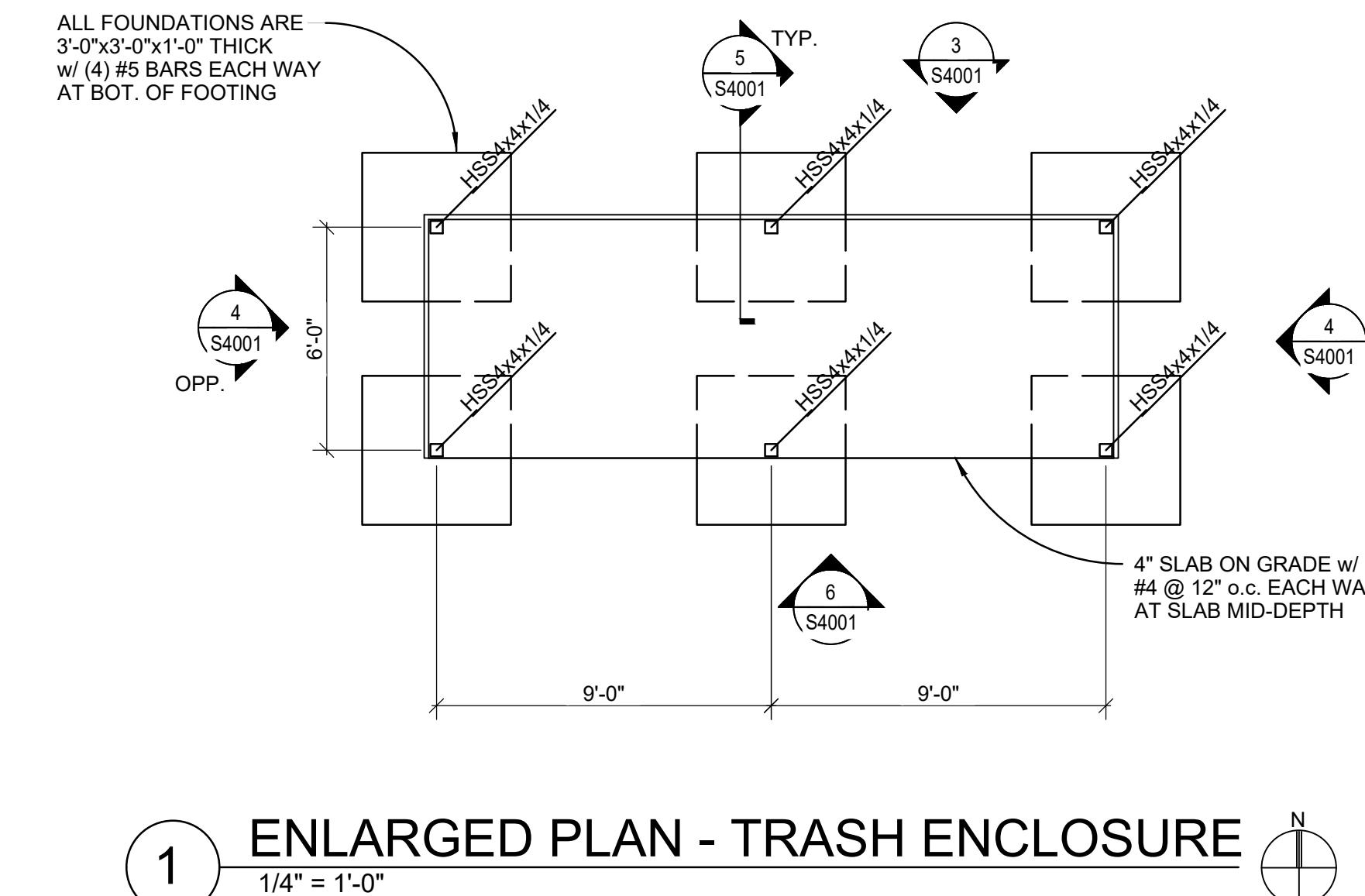


5 TRASH ENCLOSURE SECTION  
1" = 1'-0"

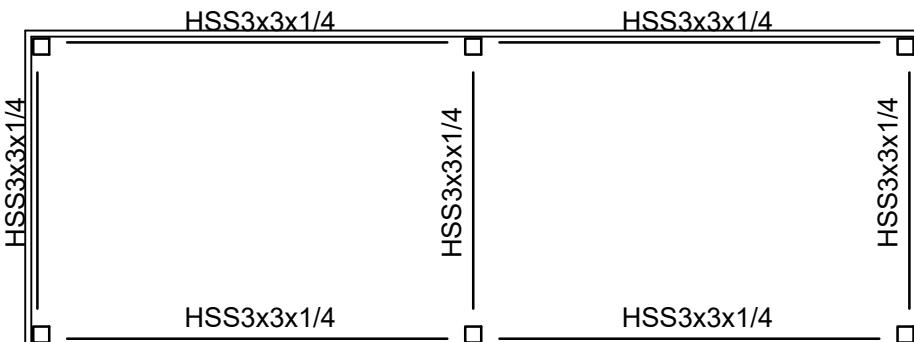


6 SOUTH ELEVATION  
1/4" = 1'-0"

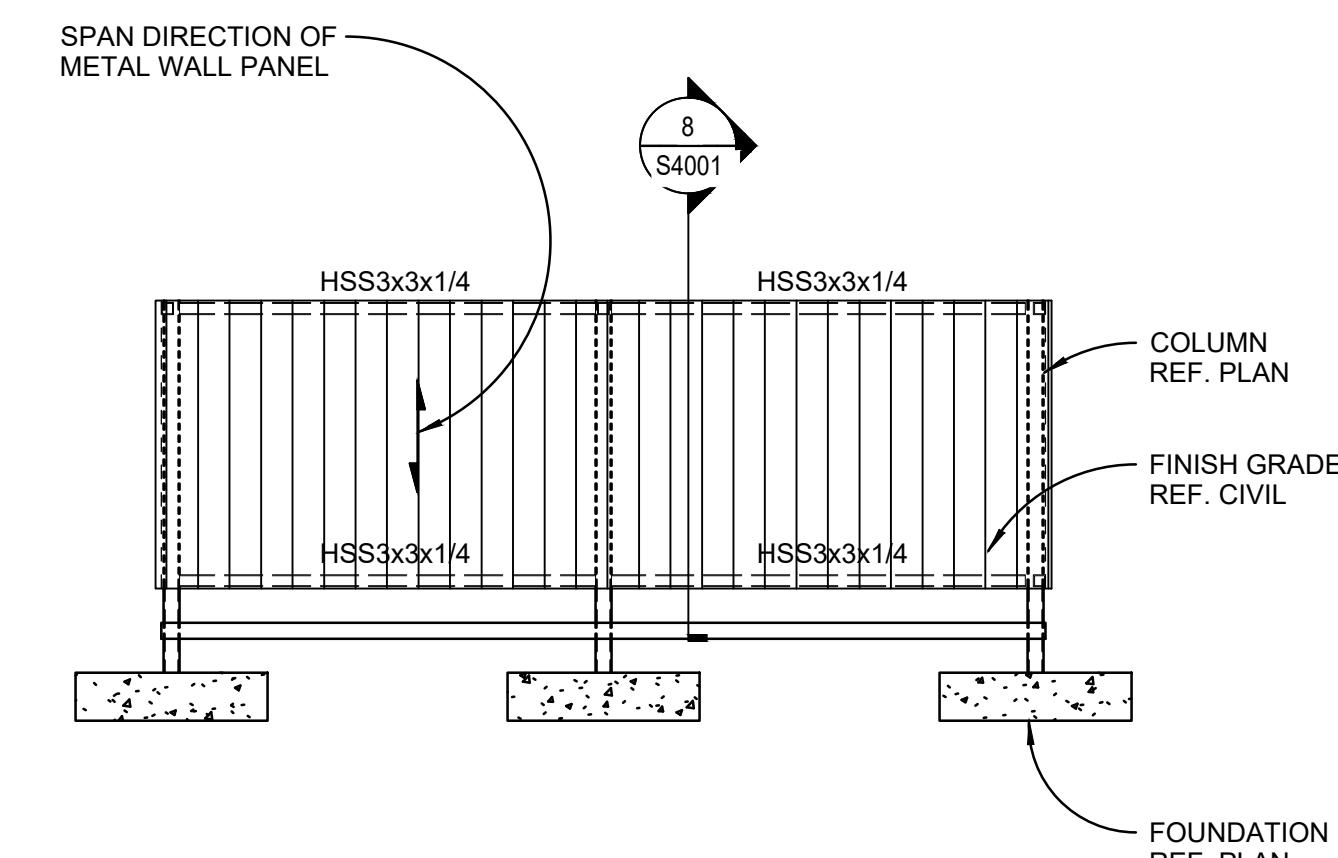
8 ENCLOSURE DETAILS  
1" = 1'-0"



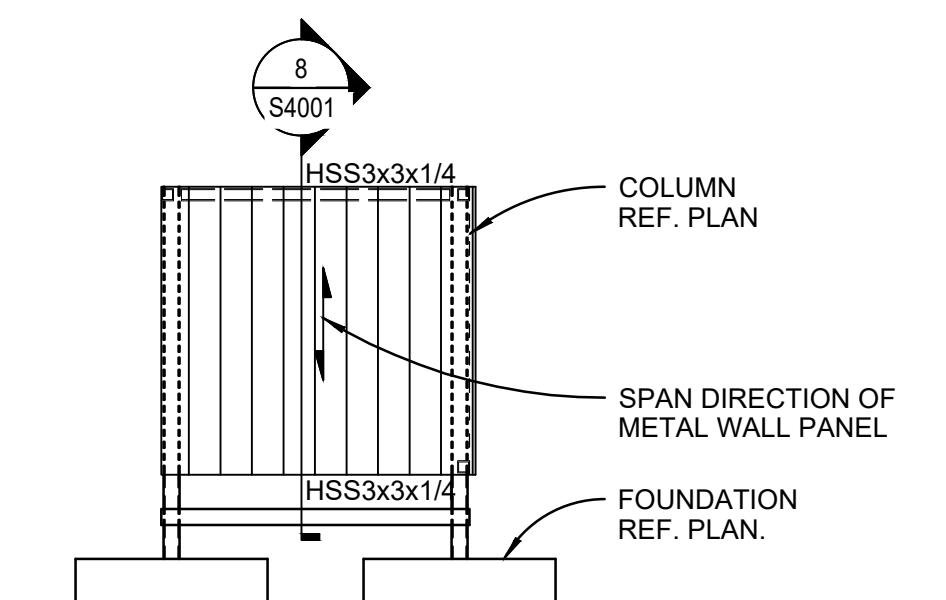
1 ENLARGED PLAN - TRASH ENCLOSURE  
1/4" = 1'-0"



2 ENLARGED PLAN - TRASH ENCLOSURE  
1/4" = 1'-0"



3 NORTH ELEVATION  
1/4" = 1'-0"



4 EAST/WEST (OPP.) ELEVATION  
1/4" = 1'-0"

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## DUMPSTER ENCLOSURE PLANS AND DETAILS

**S401**