#### Development of a 1-Dimensional Unsteady State Model of the Lower Columbia River and Application in Determining a 50% Annual Exceedance Stage Profile

RTLAND DISTRICT

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#### **Chris Nygaard**

Hydraulic Engineer Portland District May 29, 2014





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# Topics

- Purpose of Model
- 1-D HEC-RAS Model Development
  - ► Terrain
  - Model Geometry
  - Inflow Hydrograph
  - Calibration
- 50% Annual Exceedance Stage Profile Development





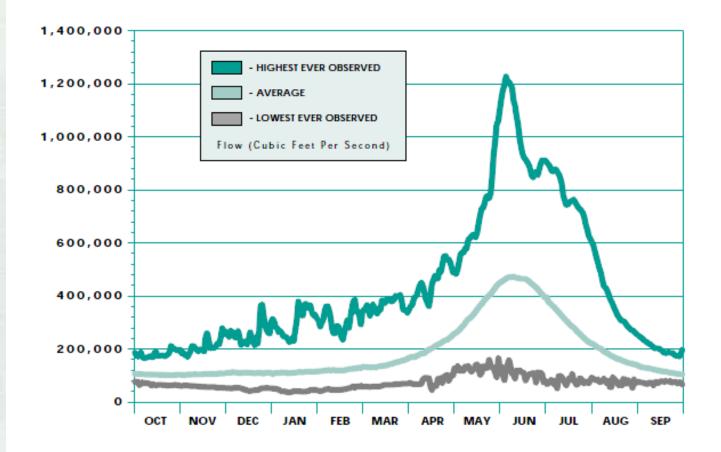
### Purpose

- Support the US Entity in the 2014-2024 Columbia River Treaty Review
- Provide hydraulic metrics associated with flood consequences
  - Inundated Areas
  - Peak Stages
  - Stage Duration

**Development was Focused on Damaging Floods** 



### Purpose



**Design Range of Flows** 



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Combined 1 meter Topographic and Bathymetric DTM

Topographic:

- 1 meter LiDAR collected 12/2009 2/2010
- 0.2% AEP inundated area coverage

Bathymetric:

- Data dates range from 1851-2010
- Quality Varies

Datum:

Albers Equal Area Conic, USGS version.

Modified to US Feet

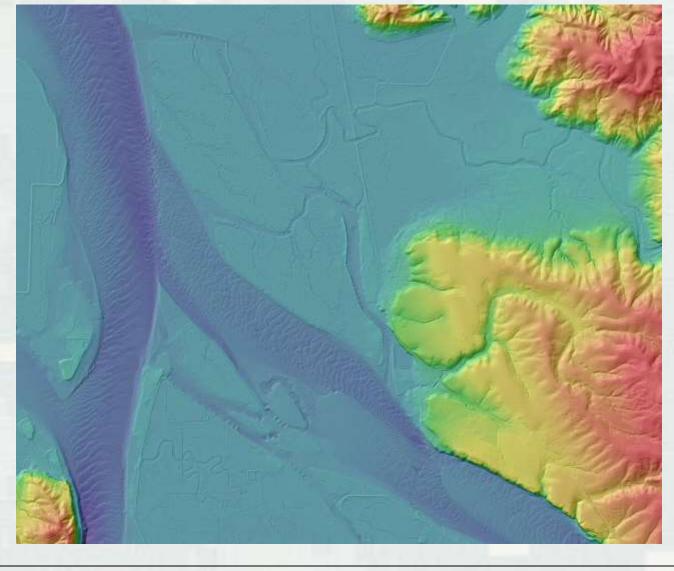
•NAVD88 (Geoid 2009), feet

Development Team:

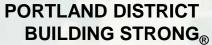
- USACE, Portland District
- David Smith and Associates, Lead
- David Evans and Associates, Sub
- CC Patterson and Associates, Sub

•Data from multiple sources: USACE, NOAA, LCREP, US BOR, and more...

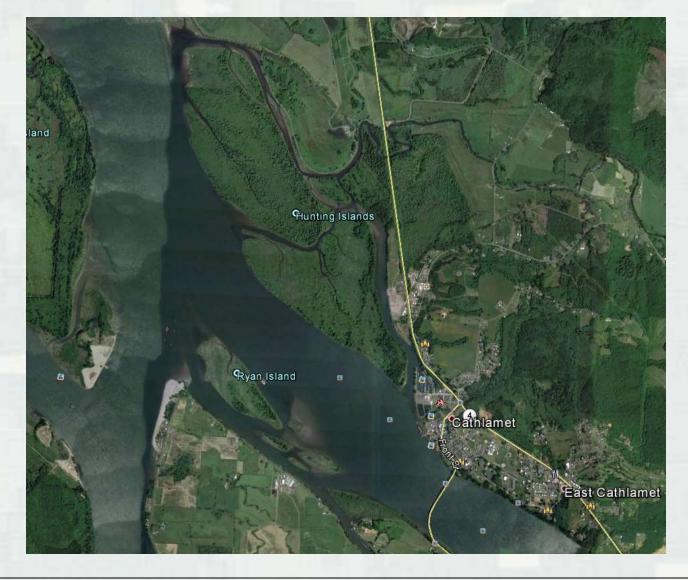








H.A.H





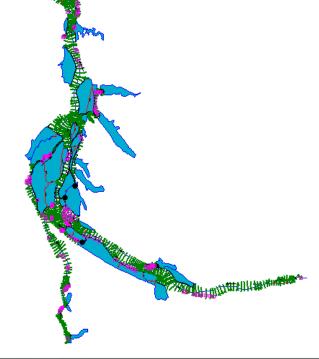


Combined 1 meter Topographic and Bathymetric DTM



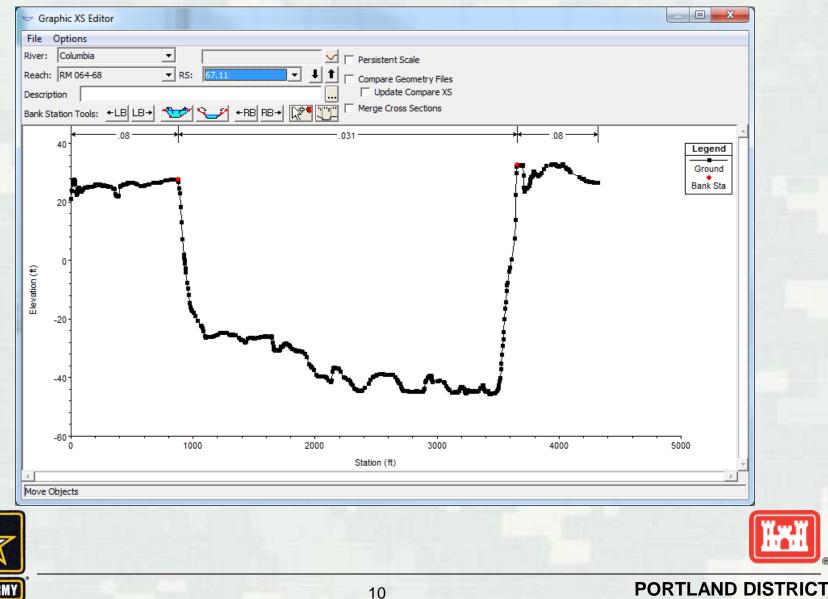
1-D Unsteady State HEC-RAS

Geometric Data Generated by HEC-GeoRAS

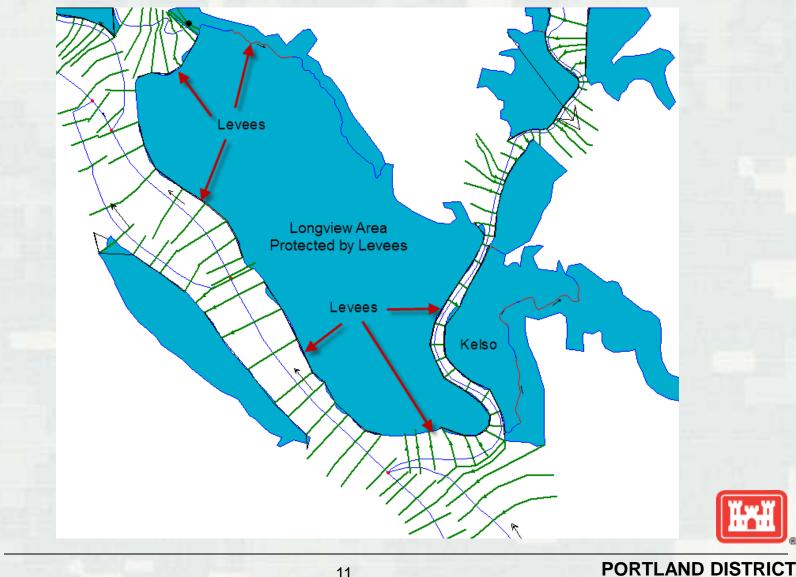






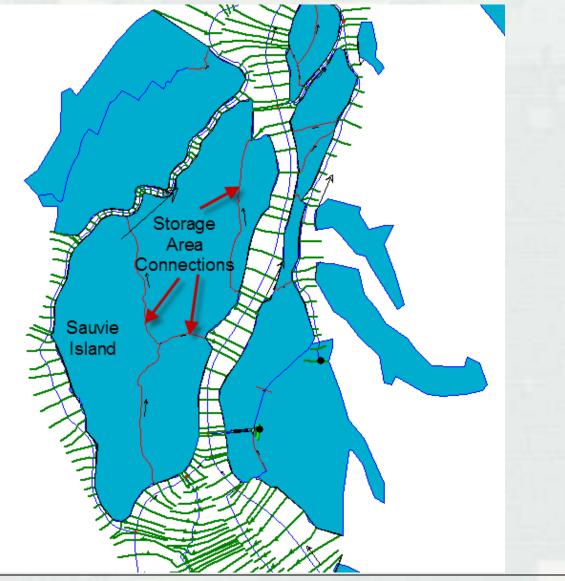


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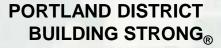
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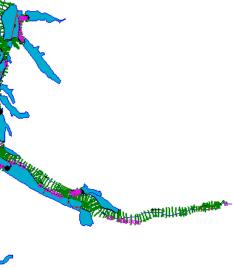


Storage Area Connection	n Breach Data		
SA Connection 424		▼ ↓ ↑ Delete this Breach Delete all Breaches ↓	
🔽 Breach This Struct	ure		
Breach Method: User E	Entered Data 💌	Breach Plot Breach Progression Simplified Physical Breach Repair (optional) Pa	ramet
Center Station:	1493	Lower Columbia CRT Plan: R1977	- 7
Final Bottom Width:	423.4		ī
Final Bottom Elevation:	8.5	30 Spillway	
Left Side Slope:	0.5	25	<u>บ</u>
Right Side Slope:	0.5		
Breach Weir Coef:	2		
Breach Formation Time (hrs): 4.68		€ 20 uotite endition 15	
Failure Mode: Pi	ping 💌		
Piping Coefficient:	0.5	10	
Initial Piping Elev:	15	5	
Trigger Failure at:	S Elev +Durat 💌		
Threshold WS	27.05	0 1000 2000 3000 4000 5000 6000	
Duration Above Thresho	ld 120	Station (ft)	-
Immediate Initiation WS	27.4		•
		OK Can	cel

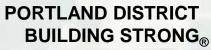




- 1449 Cross Sections
  - Average spacing ~2000 ft
- 162 Storage Areas
- 208 Lateral Structures
  - 61 with Breach Data
- 77 Storage Area Connections
  - 12 with Breach Data

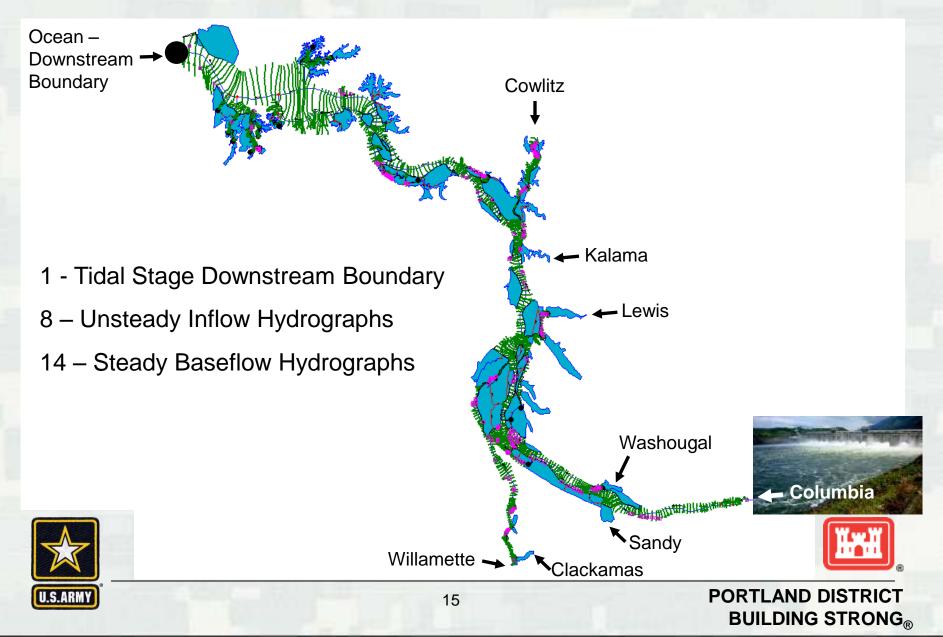




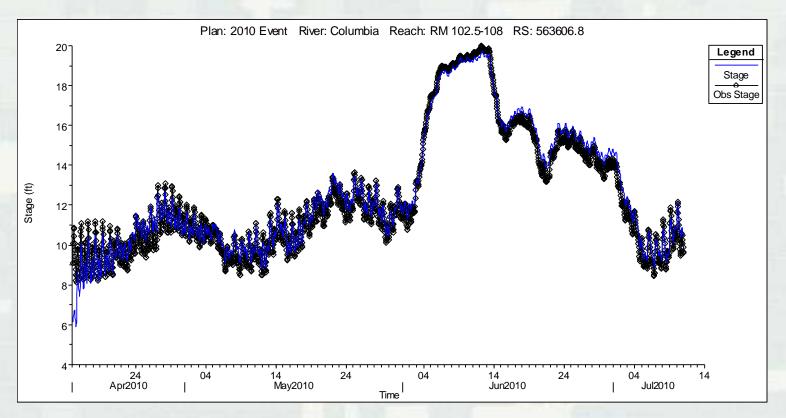




# Inflow Hydrograph Development



# **Flood Calibration**



Computed and Observed Stages on Columbia River at Vancouver for 2010 Event



Why develop the profile?

Survival Benefits Unit water surface profile was defined in Section 2., ERTG Document 2011-01; "Use the 2-year flood elevation or EHHW (mean highest monthly tide), whichever is higher".





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#### Approach:

- Develop a continuous observed inflow data set that represents the current conditions (1973-2010).
- Calibrate model to annual peak stages
- Calculate annual peak stages for period of record
- Calculate 50% AEP stage



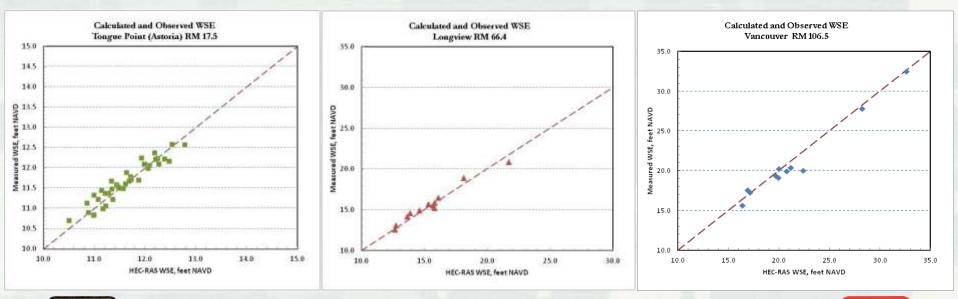


#### **Annual Peak Calibration**

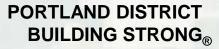
#### Tongue Point Tidally Dominated

#### Longview Tide-Flow Mixed Peaks

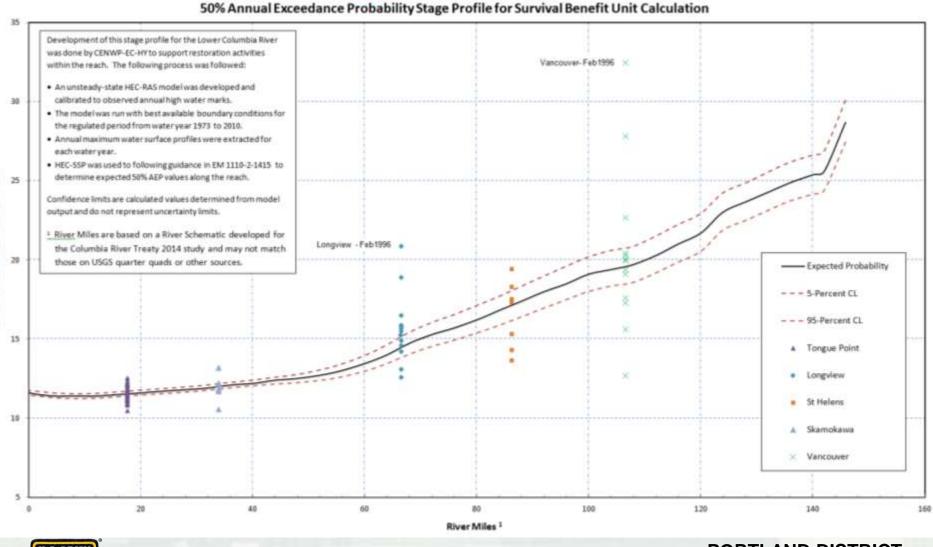
#### Vancouver Flow Dominated







**DRAFT** Lower Columbia River



Water Surface Elevation, feet NAVD

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Water Surface Elevation, feet NAVD						
	Expected	5-Percent	95-Percent			
RM <sup>1</sup>	Probability	CL	CL			
145.87	28.7	30.1	27.4			
142.04	25.6	26.9	24.4			
140.15	25.4	26.6	24.2			
136.05	24.9	26.2	23.7			
132.13	24.3	25.6	23.2			
128.19	23.7	24.9	22.5			
123.99	23.0	24.2	21.9			
120.07	21.7	22.9	20.6			
115.93	21.0	22.2	19.9			
112.10	20.3	21.5	19.2			
107.86	19.7	20.8	18.6			
104.06	19.4	20.6	18.4			
100.00	19.1	20.2	18.0			
96.06	18.5	19.6	17.5			
92.06	18.0	19.0	17.0			
88.04	17.4	18.3	16.4			
83.92	16.8	17.7	15.9			
80.65	16.3	17.2	15.5			
75.98	15.7	16.5	14.9			
72.12	15.3	16.1	14.6			
68.15	14.8	15.4	14.1			
63.99	14.0	14.6	13.5			
60.41	13.5	14.0	13.0			
55.84	13.0	13.4	12.6			
51.85	12.7	13.0	12.4			
47.70	12.5	12.7	12.2			
44.06	12.4	12.6	12.2			
40.24	12.2	12.4	12.1			
36.35	12.1	12.3	11.9			
31.89	11.9	12.1	11.8			
28.07	11.8	12.0	11.7			
23.76	11.7	11.8	11.6			
20.24	11.6	11.8	11.5			
16.35	11.5	11.7	11.4			
12.18	11.4	11.6	11.3			
8.47	11.4	11.5	11.2			
4.65	11.4	11.6	11.3			
1.70	11.5	11.7	11.4			
0.15	11.6	11.7	11.5			



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#### Acknowledgements

Gary W. Brunner, P.E., D.WRE, M.ASCE Senior Technical Hydraulic Engineer Hydrologic Engineering Center. USACE

James D. Crain Hydrologist and Hydraulic Engineer Portland District, USACE



